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VERSA-LOK®
Retaining Wall Systems
Solid Solutions.™



Square Foot

Design & Installation Guidelines

WELCOME

This guide is intended to illustrate design and construction capabilities of Square Foot™ retaining walls. There are many variables to consider, however, when planning or constructing any retaining wall. Soil types, drainage, loading, topography, and height each need to be addressed on every project to ensure safe, trouble-free installation.



Walls that support heavy loads or exceed three feet in height require special soil reinforcement and often professionally designed plans. Consult a qualified engineer if unsure about any construction, site, or soil conditions.

A variety of technical support is available including in-house engineering assistance and reference literature. The following Technical Bulletins* are for VERSA-LOK® Standard units, however, the general principles also apply to Square Foot units.

Please call **(800) 770-4525** with questions or to request any of the following:

- **Technical Bulletin #1** *Shoreline, Waterway and Retention Pond Protection*
- **Technical Bulletin #3** *Curves and Corners*
- **Technical Bulletin #4** *VERSA-LOK® Caps*
- **Technical Bulletin #5** *Base Installation*
- **Technical Bulletin #7** *Tiered Walls*
- **Technical Bulletin #8** *Fences, Railings, and Traffic Barriers*
- **Technical Documentation for VERSA-Grid® Soil Reinforcement**

**Technical Bulletins are also available for download online at www.versa-lok.com.*

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INTRODUCTION & UNIT SPECIFICATIONS

1

Square Foot™ retaining walls are a permanent, attractive, preferred alternative to ordinary retaining wall types. They have a natural split-face texture to complement any environment and because they are made of concrete, are environmentally safe.



Square Foot retaining walls are economically installed without mortar and do not require concrete footings. In addition, each unit covers one square foot of wall face, making Square Foot retaining walls an economical choice for large commercial and agency projects.

Square Foot retaining walls have rapidly earned approval from architects, engineers, and contractors. It may be easily installed by contractors, grounds maintenance

personnel, or municipal construction crews. Matching concrete caps are available to attractively finish any Square Foot retaining wall.

Square Foot units are routinely used by many state transportation departments. Properly designed, walls may be constructed to heights in excess of 40 feet.

Square Foot units are made from high-strength, low-absorption concrete on standard block machines. Holes and slots molded into units accept VERSA-TUFF® non-corrosive, nylon/fiberglass pins. Pins interlock units and help provide consistent alignment. This unique hole-to-slot pinning system permits easy variable-bond construction—keeping vertical joints tight.

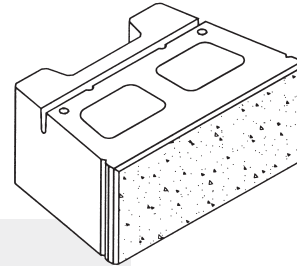
1

INTRODUCTION & UNIT SPECIFICATIONS

SQUARE FOOT™ UNIT

(Actual unit size and weight may vary slightly by region.)

Height:	8 inches	(203.2 mm)
Width (face):	18 inches	(457.2 mm)
Width (rear):	14 inches	(355.6 mm)
Depth:	12 inches	(304.8 mm)
Face Area:	1.0 foot ²	(0.093 m ²)
Weight:	87 lbs.	(39.46 kg)



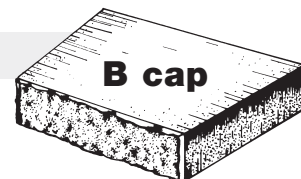
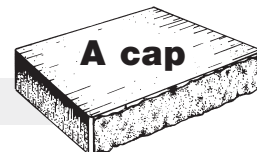
VERSA-TUFF® PIN

Length:	6.8 inches	(172.7 mm)
Diameter:	0.48 inches	(12.2 mm)
Material:	Glass-Reinforced Nylon	



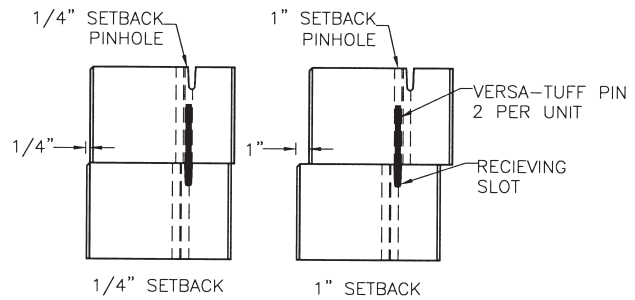
CAP UNITS

Height:	3-5/8 inches	(92.1 mm)
Width (face):	14 inches	(355.6 mm)
Width (rear):	A cap 12 inches	(304.8 mm)
	B cap 16 inches	(406.4 mm)
Depth:	12 inches	(304.8 mm)
Weight:	A cap 40 lbs.	(18.14 kg)
	B cap 50 lbs.	(22.68 kg)



Pinning

Square Foot™ units interlock with non-corrosive VERSA-TUFF® pins (two per unit). As wall courses are installed, pins are inserted through holes in uppermost course units and are received in slots of adjacent lower course units. Receiving slots allow pinning for near vertical (1/4" setback) or canted (1" setback) walls.

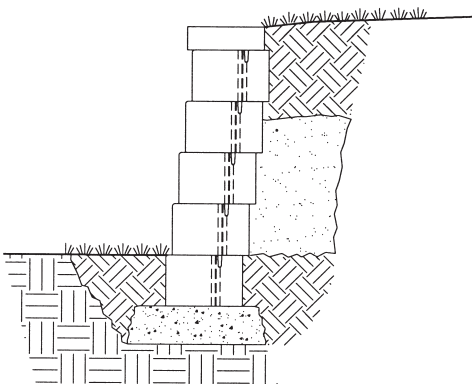


PINNING DETAIL

Unreinforced Walls

For shorter walls, Square Foot retaining walls work purely as gravity systems—unit weight alone provides resistance to earth pressures. Batter setback of wall faces offers additional resistance against overturning.

Maximum allowable wall height for gravity walls varies with soil and loading conditions. Generally, with level backfill, good soils and no excessive loading, Square Foot gravity walls are stable to heights of three feet.



Reinforced Walls

When weight of units alone is not enough to resist soil loads, horizontal layers of geosynthetics are used to reinforce soil behind walls. With proper soil reinforcement and design, Square Foot retaining walls can be constructed to heights in excess of 50 feet. Geosynthetics and soil combine to create reinforced soil structures that are strong and massive enough to resist forces exerted on them.

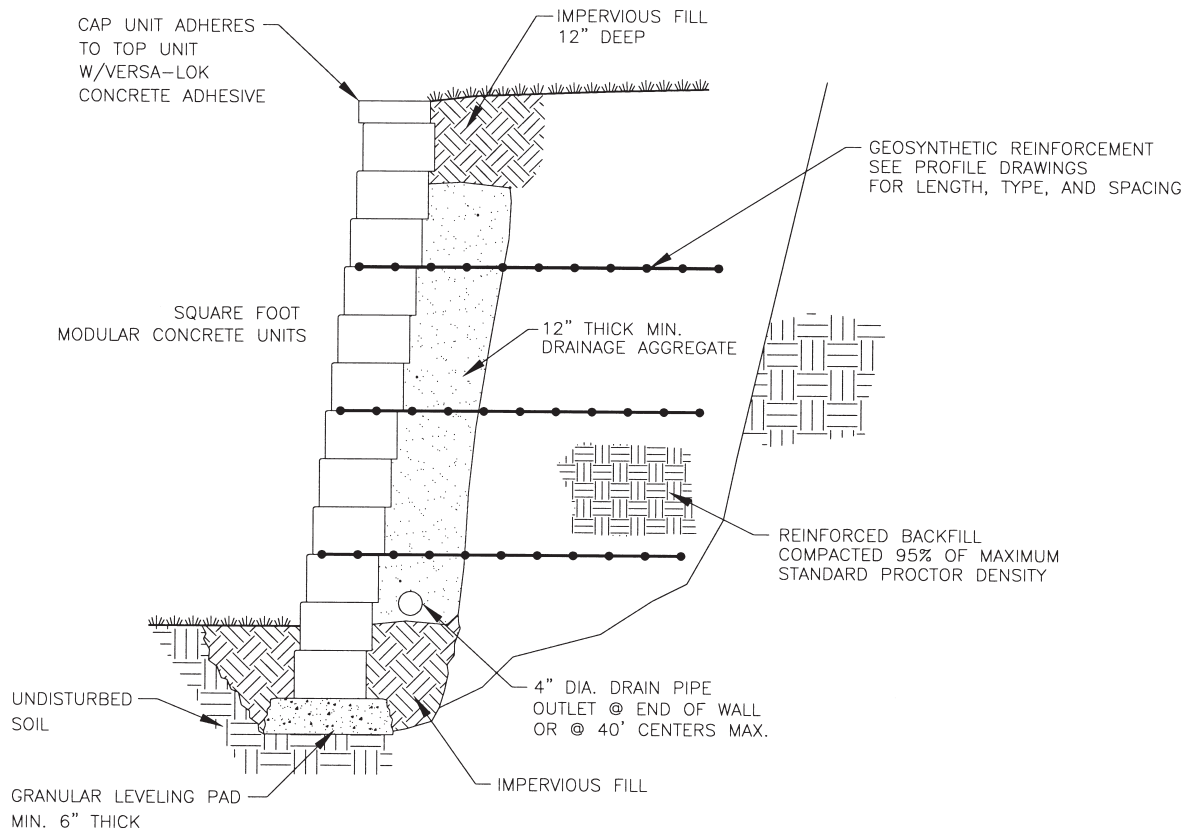
2

SYSTEM OVERVIEW

Reinforced Wall Typical Section

(The need for design of soil reinforcement and drainage materials is site/soil dependent.)

A Square Foot™ retaining wall has a number of components: the Square Foot modular concrete facing units, geosynthetic reinforcement (if required), leveling pad, backfill, drainage features, and retained soil.



Foundation

Foundation soils on which walls will rest must be stiff, firm, and have sufficient capacity to support wall system weight. Any loose, soft, or compressible material must be removed and replaced with properly compacted backfill. The bearing capacity of the foundation soils should be addressed by a soils engineer.

Square Foot™ retaining walls are installed on leveling pads consisting of well-graded angular gravel. The most commonly used material for leveling pads is that which is used locally as road base aggregate. Granular leveling pads provide stiff, yet somewhat flexible, bases to distribute wall weights.

Rigid concrete footings extending below frost are not required or recommended. Because Square Foot units are installed without mortar, they are free to move slightly in relation to each other. Flexibility of the leveling pad and wall units accommodates freeze/thaw cycles without damage to structures. Square Foot walls, installed on granular leveling pads, have been successfully used on projects throughout North America—including shoreline applications and walls exceeding 50 feet in height.

If a contractor chooses to form leveling pads using concrete, unreinforced pads should be made of lean concrete mix (200-300 psi) and no more than two inches thick. To ensure correct Square Foot unit alignment, special care needs to be taken to construct concrete pads that are exactly level. In rare situations where rigid, reinforced-concrete footings are required, they should be placed below seasonal frost depths.

Embedment

Square Foot retaining walls usually have one-tenth of exposed wall heights embedded below grade. For example, a wall with 20 feet of height exposed above grade would have a minimum of 2.0 feet buried below grade—making a total wall height of 22.0 feet. Embedment should be increased for special conditions such as slope at the toe of walls, soft foundation soils, or shoreline applications. Embedment provides enhanced wall stability and long-term protection for leveling pads.

3

WALL COMPONENTS

Soils and Compaction

With proper design, segmental walls can be constructed within a wide variety of soil conditions. Granular soils are preferred as fill in the areas reinforced with geosynthetics. However, fine-grained soils such as clays are acceptable, too. Usually, coarse soils require less soil reinforcement and are easier to compact than fine soils. Problem materials like expansive clays, compressible soils, or highly organic soils (top soil) should be avoided or properly addressed in designs.

Proper compaction of foundation and backfill soil is critical to long-term performance of retaining wall systems. Loose backfill will add pressure on walls, collect water, cause settlement, and will not anchor soil reinforcement materials properly.

Foundation and backfill materials should be compacted to at least 95 percent of standard Proctor density. (Proctor density is the maximum density of the soil achieved in a laboratory using a standard amount of compaction effort.) Generally, construction observation and testing for proper soil type and compaction is provided by the project's soils engineer.

Drainage Within Walls

Segmental retaining walls are designed assuming no hydrostatic pressure behind walls. Drainage aggregate (angular gravel, clear of fines) placed behind walls helps eliminate water accumulation. Because no mortar is used in Square Foot™ wall construction, water is free to weep through joints of installed units. For walls taller than three feet high, a perforated drain pipe is recommended at the base of the drainage aggregate to quickly remove large amounts of water.

If high groundwater levels are anticipated or if the wall is along a shoreline, additional drainage materials behind and below reinforced fill may be required. Filter fabric may be required to prevent unwanted migration of fine soil particles into the drainage aggregate.

Surface Drainage

Wall sites should be graded to avoid water flows, concentrations, or pools behind retaining walls. If swales are designed at the top of walls, properly slope them so water is removed before it can flow down behind walls.

Give special attention to sources of stormwater from building roofs, gutter downspouts, paved areas draining to one point, or valleys in the topography. Be sure to guide flows from these areas away from retaining walls. Slope the soil slightly down and away from wall base to eliminate water running along base and eroding soil.

If finish grading, landscaping, or paving is not completed immediately after wall installation, temporarily protect the wall from water runoff until adjacent construction and drainage control structures are completed.

Geosynthetic Reinforcement

Geosynthetics are durable, high-strength polymer products designed for use as soil reinforcement. Horizontal layers of geosynthetic provide tensile strength to hold the reinforced soil together, so it behaves as one coherent mass. The geosynthetic reinforced soil mass becomes the retaining wall. Sufficient length and strength of geosynthetics can create a reinforced soil mass large enough and strong enough to resist destabilizing loads. Geosynthetic layers also connect the Square Foot units to the reinforced soil.

Geosynthetics are made from several types of polymers that resist installation damage and long-term degradation. Geosynthetics are designed to interact with the soil for anchorage against pullout and resistance to sliding. Geogrids, the most common soil reinforcement for walls, are formed with an open, grid-like configuration. Geotextiles (solid fabrics) are also used. Product-specific testing determines the durability, soil interaction, and strength of each type of geosynthetic. The interaction of various geosynthetics with Square Foot units (connection strength) has also been thoroughly tested.

Geosynthetic layers must be nominally tensioned and free of wrinkles when placed. Geosynthetics are generally stronger in one direction—the roll direction. It is important that the high-strength direction is placed perpendicular to the wall face, in one continuous sheet (no splices). Along the wall length and parallel to the face, adjacent sections of reinforcement are placed immediately next to each other without overlap to create 100-percent coverage with no gapping, and with special details for curves and corners.

The required type, length, vertical spacing, and strength of geosynthetic vary with each project depending on wall height, loading, slopes, and soil conditions. A professional Civil Engineer (P.E.) should prepare a final, geogrid-reinforced wall design for each project.

4

ENGINEERING

VERSA-LOK® Square Foot™ walls are designed as traditional gravity walls. For unreinforced walls, the stabilizing weight of the battered wall units is compared to the loading on the walls to ensure stability against overturning and sliding (page 12, Figure 1A). When the loading exceeds the stability of the units alone, a larger gravity mass is created from reinforced soil (page 12, Figure 1B).

Loading on segmental walls is dependent on soil conditions, surcharges, slopes, water conditions and wall heights. Accurate knowledge of each of these properties is needed for a proper design. Soil properties required for a segmental retaining wall design include the internal friction angle (ϕ) and soil unit weight (γ). Generally, the cohesion (c) of any fine-grained soils is conservatively ignored to simplify the design.

To ensure stability of a reinforced retaining wall, the wall engineer must design the reinforced soil mass large enough to resist loads from outside the wall system (external stability) and with enough layers of proper strength geosynthetic to keep the reinforced soil mass together (internal stability). In addition, the design must have sufficient geosynthetic layers to keep units stable and properly connected to the reinforced soil mass (facial stability).

For internal stability, the wall designer can address potential overstress by using a higher strength geogrid or adding more geogrid

layers by reducing vertical space between geogrid layers. Potential pullout or internal sliding concerns can be addressed by lengthening the geogrid layers.

Internal compound stability is the potential for compound failures starting directly behind the wall, passing through the reinforced soil mass and exiting out the front face of the wall. The wall design engineer can address internal compound stability by using a higher strength geogrid type, adding geogrid layers, lengthening geogrid layers or improving the reinforced soil type.

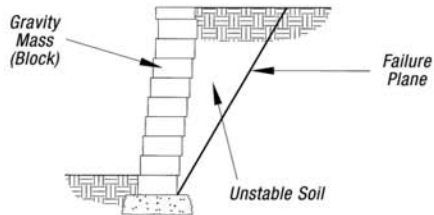
For facial stability, the wall design engineer can address connection concerns by adding geogrid layers (including shorter supplementary layers) or using a higher connection strength geogrid.

For external stability, potential overturning or sliding both can be addressed by lengthening the geogrid layers to create a larger, more stable reinforced soil mass.

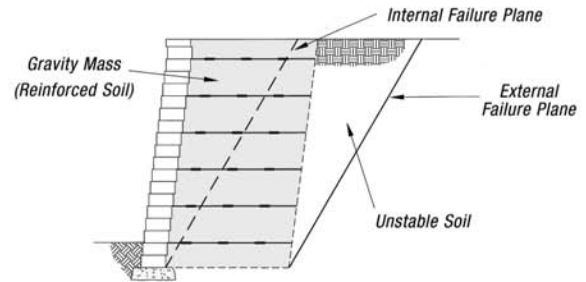
Evaluation of geotechnical concerns generally is the responsibility of the soils engineer. However, in some cases, these can be addressed by lengthening and strengthening the geogrid layers beyond what is required for the structural wall design.

Stability Analysis

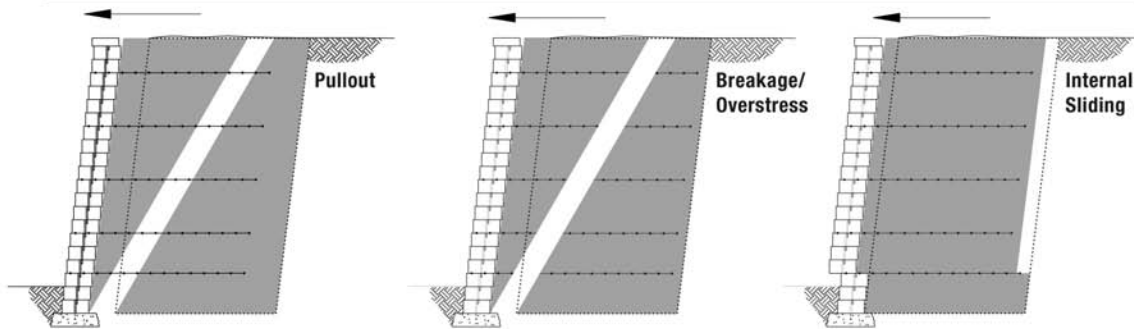
Unreinforced Wall (Figure 1A)



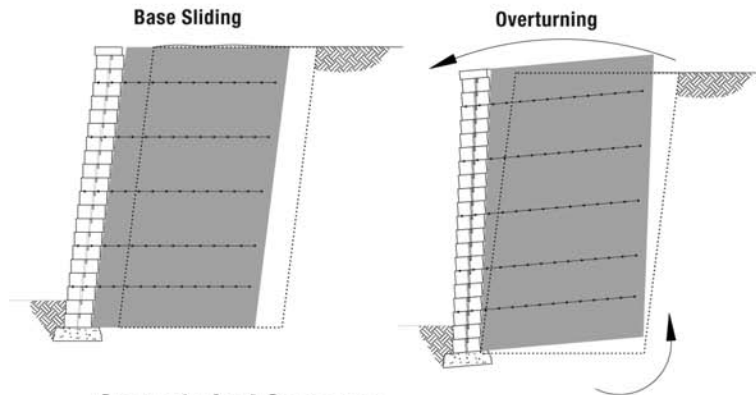
Reinforced Wall (Figure 1B)



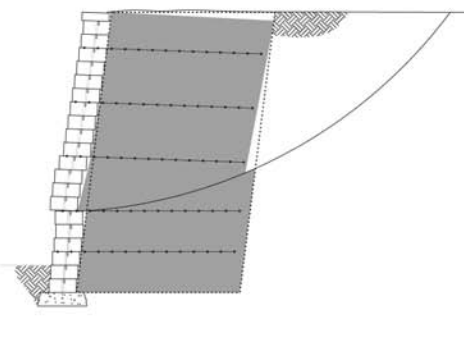
Internal Stability



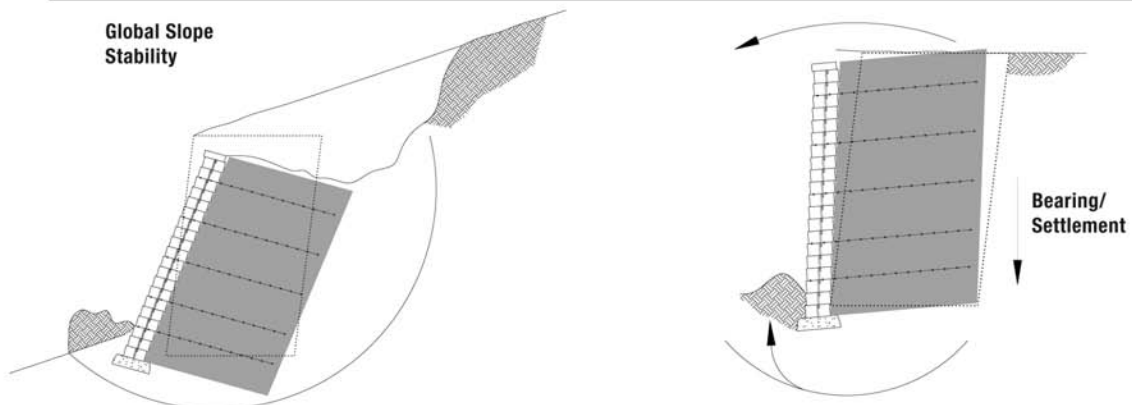
External Stability

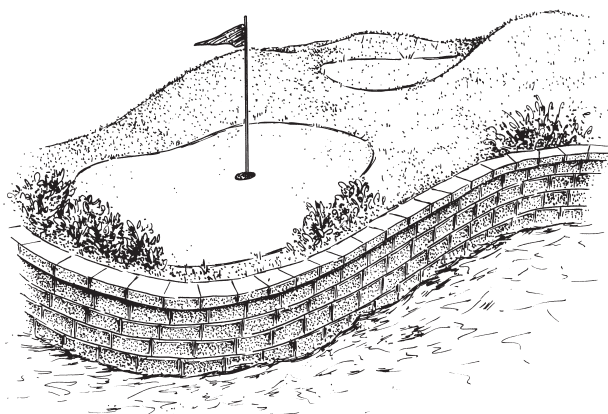


Internal Compound



Geotechnical Concerns



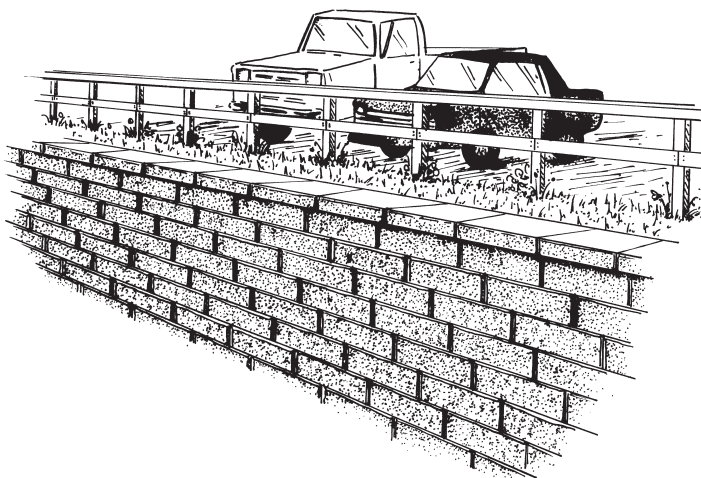


Shorelines

Square Foot™ retaining walls perform well in shoreline applications. However, special design considerations are often necessary to ensure that water pressures do not build up behind walls. Special provisions may include granular reinforced backfill, additional drainage aggregate, drainage behind reinforced soil masses, and filter fabric. Protection of bases from water scour, wave action, and ice may also be necessary.



See VERSA-LOK® Technical Bulletin #1 for more information regarding shoreline and retention pond protection.



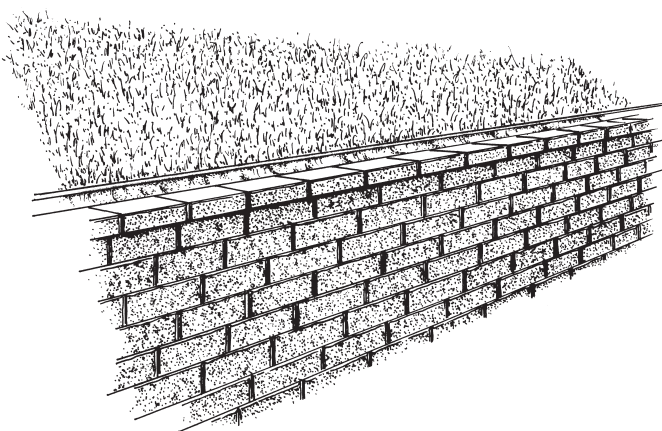
Loads Behind Walls

Surcharge loads behind walls can substantially increase amounts of required soil reinforcement. Common surcharge loads include: parking areas, driveways, roads, and building structures. For design purposes, permanent loads like buildings are considered to contribute to both destabilizing and stabilizing forces acting on walls. Dynamic forces like vehicular traffic are considered to contribute to destabilizing forces only.

Often, the highest surcharge loads are caused by grading or paving equipment during construction. Heavy equipment should be kept at least three feet behind the back of retaining wall units. Soil reinforcement designs should accommodate all anticipated surcharge loads—even if they will occur infrequently or just once.

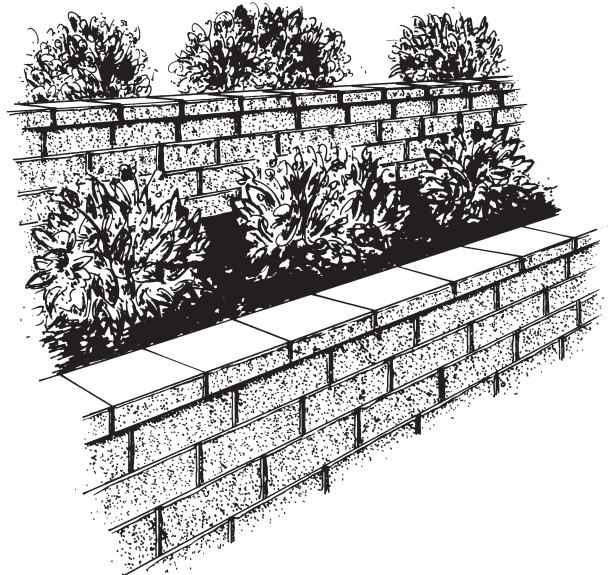
Slopes

Slopes behind walls increase pressures, sometimes doubling soil loads compared to level backfills. Steep slopes below walls can decrease stability of wall foundations. Slopes can increase the amount of soil reinforcement needed, especially the length. Generally, slopes above or below walls should be no steeper than 2:1 (horizontal:vertical).



Tiering

Aesthetically, dividing large grade changes into tiered wall sections may sometimes be desirable. However, upper wall tiers can add surcharge loads to lower walls and necessitate special designs. To avoid loading lower walls, upper walls must be set back horizontally at least twice the height of the lower walls. If walls are placed closer, lower walls must be designed to resist the load of upper walls.



! *Several closely spaced tiered walls can create steep, unstable slopes. If tiered walls make a grade change steeper than 2:1 (horizontal:vertical), global slope stability may need to be reviewed by a qualified soils engineer. See VERSA-LOK® Technical Bulletin #7 for more information regarding tiered wall construction.*

6

PLANNING, ESTIMATING, & FINAL DESIGNS

Planning

Careful planning is critical to successful projects. Prior to design, accurate information needs to be gathered including soil conditions, proposed wall heights, topography, groundwater levels, and surface water conditions. Proper permits, owner approvals, utility clearances, and temporary easements should also be obtained in advance.

Planned wall alignments should be reviewed for feasibility. Make sure that layouts account for minimum curve radii, wall setback, and area needed for geosynthetic soil reinforcement. Be sure that all wall components fit within property constraints. Verify that temporary construction excavations will not undermine foundation supports of any existing structures or utilities. Considerations should also be given to site access for equipment and materials.

Estimating

Accurately estimate and order required materials including Square Foot™ units, VERSA-TUFF® pins, cap units, VERSA-LOK® Concrete Adhesive, imported backfill, leveling pad materials, geosynthetic soil reinforcement, drainage aggregate, and additional drainage materials. See the Material Estimation Worksheet on page 28 to help determine Square Foot quantities.



For reinforced-wall projects, the VERSA-Grid® estimating charts on page 29 provide approximate amounts of geogrid soil reinforcement necessary to construct walls in various soil and loading conditions. For tall walls or complex situations, our staff engineers can prepare project-specific preliminary designs to be used for estimation purposes.

Final Designs



For walls more than four feet tall, most building codes require a final wall design prepared by a licensed Civil Engineer (P.E.) registered in that state. Square Foot™ retaining wall manufacturers have a network of licensed civil engineers who are familiar with segmental retaining wall design. These individuals are available for referrals to architects, engineers, or contractors with final wall design needs.

Final wall designs may be provided prior to putting projects out for bidding. Alternatively, wall portions of projects can be specified design/build. With design/build projects, engineers/architects provide wall layout information (line and grade) but not final engineering for the wall. Contractors submit bids based on this layout including estimated labor, materials, and final engineering costs. Contractors who are awarded projects retain licensed engineers to prepare final wall designs and submit shop drawings for approval from project engineers/architects.

As with all proposed construction, a soils report prepared by a qualified geotechnical engineer is required to provide adequate information for proper design. The soils report should address overall stability of planned grade changes and allowable bearing capacity of foundation soils. The report should also include information about reinforced and retained soil properties.

For assistance in specifying, designing and engineering Square Foot retaining walls, standard design/build specifications and sample construction details are provided on pages 30 to 41. This information is also available in electronic format on a Square Foot Construction Details disk. Technical staff is also available to assist with planning, layout, preliminary engineering, and referrals for final engineering.

7

WALL CONSTRUCTION

Tools

The following tools may be helpful during construction of Square Foot™ retaining walls.

Safety Protection

Shovel

Four-Foot Level

Smaller Level

Four-Pound Sledge Hammer

Masonry Chisel

Brick Hammer

Tape Measure

Hand Tamper

Vibratory-Plate Compactor

Caulking Gun

String Line

Finishing Trowel

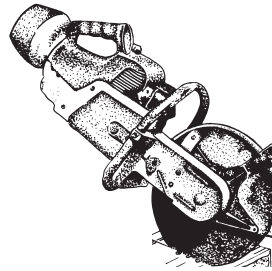
Broom

Diamond-Blade Concrete Saw

Transit or Site Level

Backhoe or Skid-Steer Loader

Unit Modification



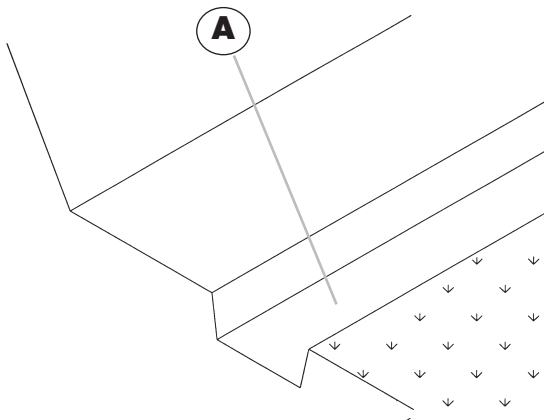
Saw-cuts are normally made using a gas-powered cut-off saw with a diamond blade.

To cut a unit, mark desired path of cut on all unit sides. Stand the unit with its face up and cut two to three inches deep along the front path. Reposition the unit and complete the cut by cutting the remainder of the unit along top and bottom paths. Cutting first into the front face ensures a straight, square edge which will likely have to fit closely next to the straight edge of an adjacent unit. Remember to always wear proper safety protection when performing cutting operations.

Excavation

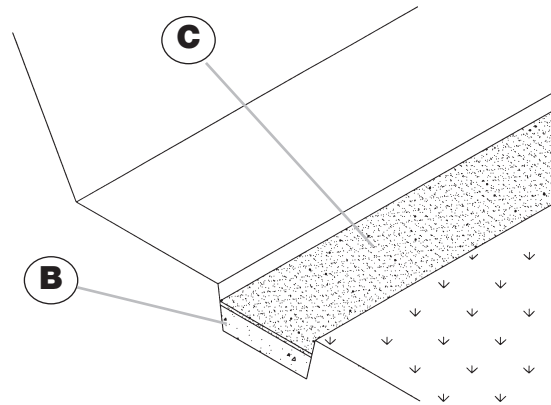
Excavate just deeply enough to accommodate the leveling pad (usually six inches) and required unit embedment below grade. **(A)** When necessary, also excavate areas where geosynthetic soil reinforcement will be placed. Required unit embedment varies with wall height and site conditions. Generally, if grade in front of the wall is level, one-tenth of the exposed wall height should be buried (embedded) below grade. Additional embedment may be required for special conditions including slopes in front of walls, soft foundation soils, and water applications.

Compact soil at the bottom of excavation. Do not place wall system on loose, soft, wet, or frozen soil—settlement may result. If the wall will sit on a previously backfilled excavation such as a utility line trench, be sure the entire depth of existing backfill is well compacted. If necessary, over-excavate soft soils and replace with properly compacted backfill.



Leveling Pad

Place granular leveling pad material and compact to a smooth, level surface. **(B)** Leveling pad should be at least six inches thick and 24 inches wide. It should consist of 3/4-inch crushed gravel. Use a thin layer of fine sand on top of the leveling pad for final leveling. **(C)**



! To quickly construct long sections of leveling pad, create forms by leveling and staking rectangular metal tubing along both sides of the planned pad. Place and compact granular material within these leveled forms and screed off excess. See VERSA-LOK® Technical Bulletin #5 and video for more tips about leveling pad construction.

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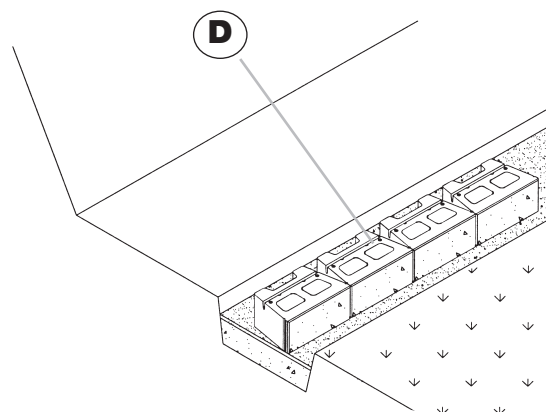
If the planned grade along wall front will change elevation, the leveling pad may be stepped in eight-inch increments to match the grade change. Always start at the lowest level and work upward.

Step the leveling pad often enough to avoid burying extra units while maintaining required unit embedment.

Base Course

Make sure that the leveling pad is level and begin placing base course units. **D** If the leveling pad is stepped, begin at the lowest course before proceeding to next course.

Align units using their backs or slot rather than their irregularly textured front faces. String lines may be helpful when aligning straight walls. Refer to pages 24 and 25 for tips on curve and corner alignment. Place units on the leveling pad, side-by-side. Front faces on adjacent units should fit tightly and unit bottoms should contact the leveling pad completely. Using a four-foot level, level units front-to-back, side-to-side with adjacent units. Tap high points with a mallet or hand tamper until level. Take time to ensure a level base course. Minor unevenness in the base course will



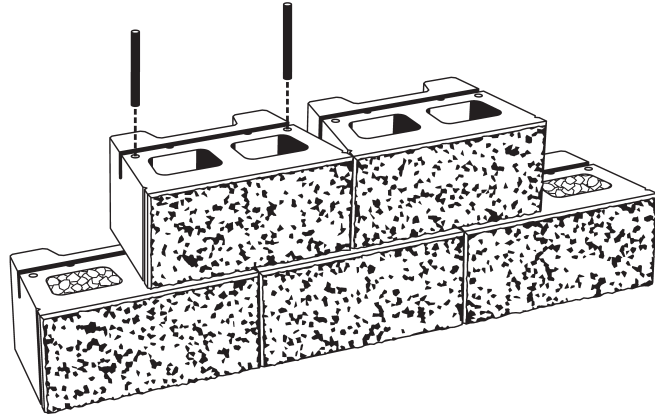
be amplified and difficult to correct after several course have been installed.

After positioning the base course, fill the voids (cores) of the units with crushed gravel. To avoid clogging the pinning slots, use gravel that is 1.0 to 1.5 inches in diameter. Crushed gravel is suggested in the base course core fill because free-draining aggregate core fill may allow water to flow down to the base. Place and compact soil backfill behind the units. Also replace and compact over-excavated soil in front of the units. The backfill behind and in front of embedded units should consist of soil—do not use drainage aggregate.



Additional Courses

Sweep off tops of installed units to remove any debris that may interfere with additional courses. Place the next course of units by setting them a short distance away from their final position and slide them into place. Sliding helps remove imperfections and debris from the top surface of installed units.



A unique hole-to-slot pinning system allows units to be installed on variable bond. (Square Foot™ units do not need to be placed exactly halfway over the two lower course units.) Vertical joints can wander in relation to other joints throughout walls. Do not try to install walls on half bond by leaving gaps in vertical face joints. Because the bond can vary, vertical face joints can and always should be tight.

Pin Square Foot units with a one-inch setback (7 degree cant) or a 1/4-inch setback (near vertical). Use the one-inch setback unless the near vertical setback is specifically required. (Near-vertical walls require more geogrid than setback walls). For the one-inch setback, use the front pin holes (outside holes). For the near vertical setback, use the pin holes within the slots (inside holes). Insert two VERSA-TUFF® pins through the appropriate holes of the

upper units into the receiving slots in the lower units. The two pins should engage two separate units in the course below. Make sure the pins are fully seated in the lower unit slots. If necessary, seat pins using a mallet and another pin. Avoid dropping pins into holes in the slots.

Pull the units forward to remove any looseness in the pin connection. Check unit alignment and levelness—adjust if necessary. If length of a course must fit into a limited space or if vertical joints begin to line up with joints below, adjust by installing partial units. Create partial units by saw-cutting whole units into pieces at least four inches wide.

Fill the voids (cores) of each course of units with angular, free-draining gravel before stacking the next course. Stack no

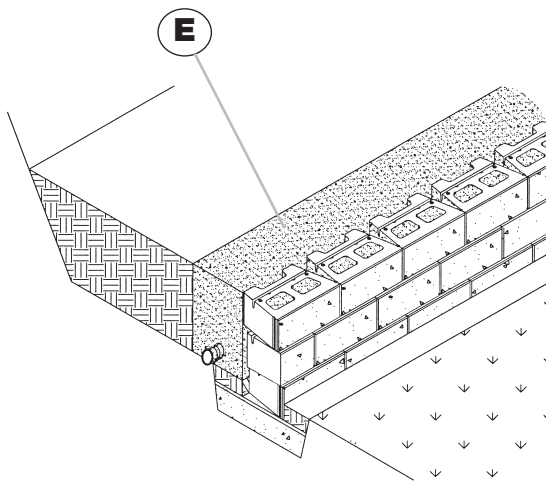
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WALL CONSTRUCTION

more than two courses before backfilling. If units are stacked too high, they may push out of alignment during placement of backfill.

Drainage Aggregate

Beginning at the level of planned grade in front of the wall, place drainage aggregate (1-1.5-inch clear, free-draining, angular gravel) between and directly behind units to a minimum of 12 inches. Drainage aggregate must be free of fine dirt or soil. Do not place drainage aggregate behind units that will be embedded. **E**



Drainage aggregate is critical to wall performance because it keeps water pressures from building up behind the wall face.

For walls over three feet in height, perforated drain pipes should be used to collect water along the base of the drainage aggregate. Drain pipes help to quickly remove large amounts of water.

For some projects, especially shoreline applications, a geosynthetic filter fabric may be required behind the drainage aggregate. Filter fabric prevents soils or sands (fines) from migrating into the drainage aggregate and wall face joints.

Compacted Soil Backfill

Proper placement and compaction of backfill is critical to the stability of a segmental wall. Poorly compacted backfill puts extra pressures on a wall—especially when it becomes wet.

Place soil backfill beginning directly behind drainage fill in layers (lifts) no thicker than six inches. Compact soil backfill—making sure that backfill is neither too wet nor dry. The amount and type of effort needed for adequate backfill compaction varies with soil type and moisture content. Generally, hand-operated vibratory-plate compactors can be used to achieve adequate compaction of granular soils—even on big projects. Fine soils such as clays should be compacted with kneading-type equipment like sheepfoot rollers.



To avoid pushing wall units out of alignment, do not use heavy self-propelled compaction equipment within three feet of the wall face.

At the end of the day's construction, protect the wall and the reinforced backfill from possible rainstorm water damage. Grade the soil backfill so water will run away from wall face and direct runoff from adjacent areas away from project site.

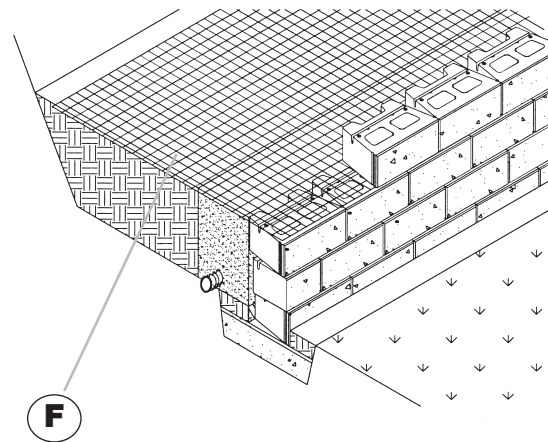
Geosynthetic Soil Reinforcement

Geosynthetic soil reinforcement, like VERSA-Grid®, is used to reinforce soil backfill when weight of Square Foot™ units alone is not enough to resist soil pressures. Soil reinforcement type, length, and vertical spacing will vary for each project and should be specified in a final wall design prepared by a licensed Civil Engineer (P.E.).

Prepare to install soil reinforcement materials by placing Square Foot units and backfilling up to the height of the first soil reinforcement layer specified on construction drawings. Lay soil reinforcement horizontally on top of compacted backfill and Square Foot units. **(F)** Geosynthetic layers should be placed about one inch from the front of Square Foot units.

Geosynthetics are usually stronger in one direction. It is very important to place them in the correct direction.

The strongest direction of the geosynthetic must be perpendicular to the wall face. For correct orientation, follow the geosynthetic manufacturer's directions carefully.

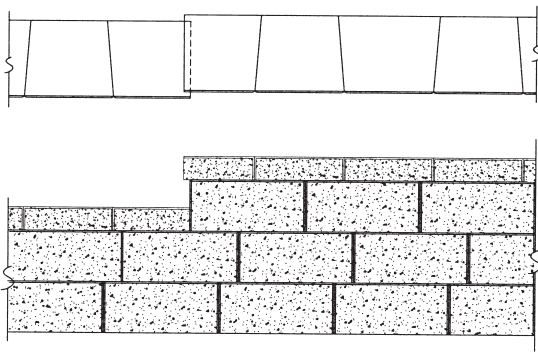


To ensure stability during construction, vertical spacing between geosynthetic layers should never exceed two feet. See VERSA-Grid® estimating charts on page 29 for assistance with preliminary material estimating.

After positioning soil reinforcement, place the next course of Square Foot units on top of soil reinforcement. Insert pins through Square Foot units and into lower course units. Place drainage aggregate against back of the units and on top of soil reinforcement. Remove slack by pulling soil reinforcement away from the wall face and anchoring at back ends.

Caps

Finish the wall by placing cap units along the top. Two cap unit types are available—A and B. Alternate A and B caps on straight walls. Use A caps for convex (outside) curves. Use B caps for concave (inside) curves.



If cap layout does not exactly match the wall radius, adjust spacing at the back of the caps—do not gap caps at the front. To completely eliminate gapping, it may be necessary to saw-cut sides of cap units.

Front faces of caps may be placed flush, set back, or slightly overhanging faces of Square Foot™ units. It is preferred to overhang cap units approximately 3/4-inch to create an “eyebrow” on top of the wall. Overhanging cap units will create a small shadow on wall units and help hide minor imperfections in wall alignment.

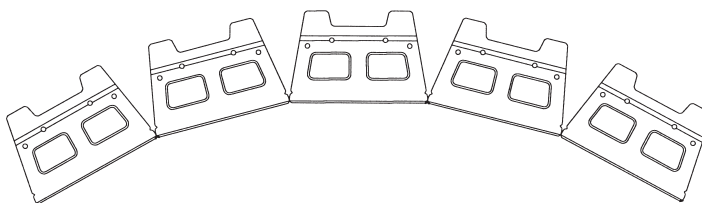
! All cap units should be arranged before securing with VERSA-LOK® Concrete Adhesive. Secure caps by placing two, continuous, 1/4-inch beads of adhesive along the top course of wall units. Set caps on prepared wall units. Do not secure caps using mortar or adhesives that become rigid. A Square Foot wall may move slightly (especially in areas subject to freeze/thaw cycles) causing a rigid cap adhesive to fail. Do not place caps if the units are too wet for the adhesive to stick. In cold weather, keep the adhesive tubes warm until just prior to use. For more information about capping, see VERSA-LOK Technical Bulletin #4.

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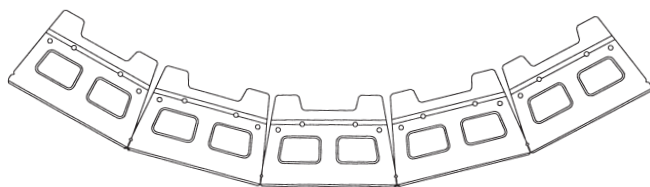
WALL DESIGN ELEMENTS

Curves

The trapezoidal shape of Square Foot™ units permits construction of concave and convex curves. Construct curves by increasing or decreasing spaces between the backs of the units—always keeping the front joints tightly aligned. The recommended minimum radius for both convex (outside) and concave (inside) curves is 4' 6" (4.5 feet), measured from the front face of the units.



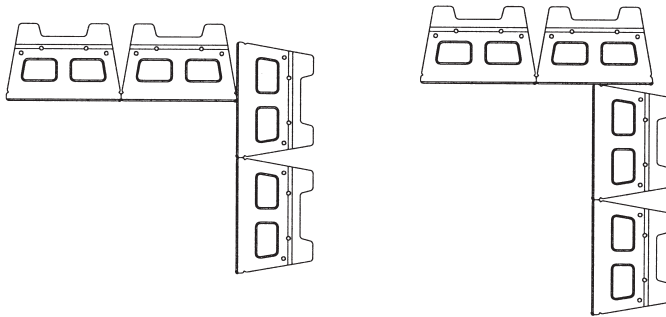
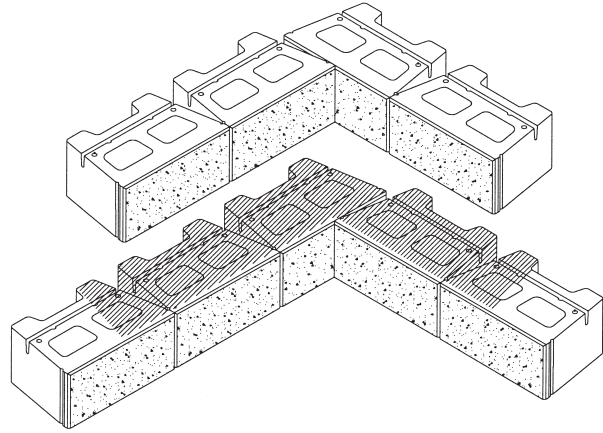
See VERSA-LOK® Technical Bulletin #3 for proper placement of geosynthetic soil reinforcement.



For convex curves, plan ahead to make sure the upper courses do not get tighter than the minimum radius. Because each course sets back, the radius of a convex curve becomes smaller as each course is stacked. To calculate minimum base course radius of a convex curve, add up the setback (either 1/4 or 1 inch) for each course needed. For example, minimum base course radius for a wall that will have six 1-inch setbacks (including embedded units) will be $(6 \times 1'') + 4'6'' = 5.0''$.

Corners

Inside 90-degree corners can be easily installed with no unit modification. For the first course, abut the unit from one side of the corner against a unit on the other side that runs past the corner. On the next course, simply reverse which side abuts and which side runs long.



UPPER COURSE

LOWER COURSE

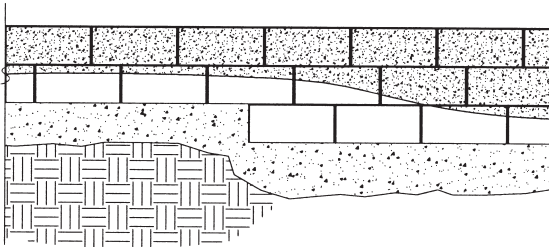
A special corner unit is needed for an outside 90-degree corner and is available only in limited markets. Check with your local Square Foot™ supplier. For ease of installation, it is suggested to replace outside corners with a tight radius convex curve whenever possible.

8

WALL DESIGN ELEMENTS

Stepped Base Elevations

If the final grade along the front of the wall changes elevation, the leveling pad and base course may be stepped in eight-inch increments to match the grade change. Always start at the lowest level and work upward.



Step the leveling pad often enough to avoid burying extra units while maintaining required unit embedment.

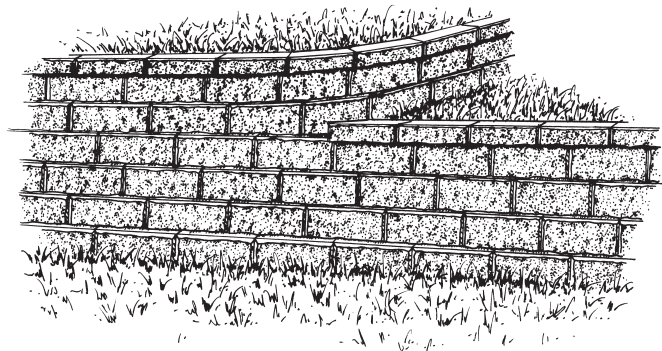
Returns

As an option to stepping wall tops, grade changes at the top of a wall can be accommodated by creating returns that turn into slopes behind a wall.

Returns create a terraced appearance instead of several small steps along the top of a wall.

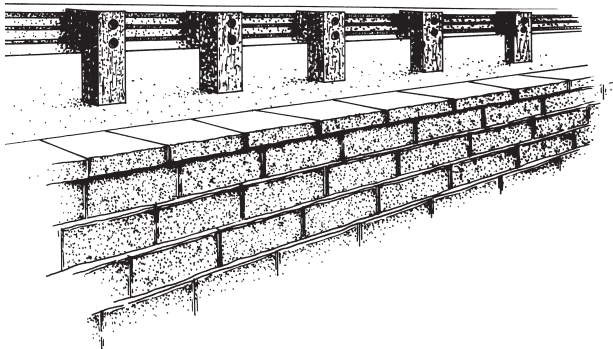


See VERSA-LOK® Technical Bulletin #7 for more information on tiered walls.



Guide Rails, Railings and Traffic Barriers

For safety purposes, a variety of barriers may be placed behind Square Foot™ walls, including fences, railings and guide rails. Barriers should be placed several feet behind wall faces to provide post foundations. Posts may penetrate geosynthetic soil reinforcement layers in accordance with the manufacturer's and engineer's recommendations.



When space is limited, properly designed, reinforced concrete barriers can be placed directly on top of walls. Expansion joints and bond breaks should be provided to accommodate differential movement between rigid barriers and flexible wall faces. Cantilevered supports extending behind walls stabilize the barriers against overturning.

! For more information about guide rails, railings, and traffic barriers see *VERSA-LOK® Technical Bulletin #8* or *Construction Details Disk*—available *FREE* by calling (800) 770-4525.

MATERIAL ESTIMATION WORKSHEET

Square Foot™ Units

Area of Wall (SF) x 1.0 Units per SF = Number of Units

_____ SF x 1.0 = _____ Units Needed

VERSA-TUFF® Pins

Units x 2 Pins per Unit = Number of Pins

_____ Units x 2 = _____ Pins Needed

NOTE: Pins will not be used on the base course.

Cap Units

Lineal Feet of Wall (LF) x .86 = Number of Caps

_____ LF x .86 = _____ Caps Needed

straight walls - use half **A** caps and half **B** caps

inside curves - use **B** caps

outside curves - use **A** caps

*Additional caps may be needed for special splits or cuts.
Gradual curves may require a combination of A & B caps.*

VERSA-LOK® Concrete Adhesive

11 oz. Tube: _____ LF ÷ 14 LF per Tube = _____ Tubes

For estimating purposes, the tables on the following page provide approximate amounts of geogrid soil reinforcement needed to construct walls in certain soil and loading conditions. For tall walls or complex situations, in-house staff engineers can prepare project specific preliminary designs to be used for estimation purposes.

VERSA-GRID® ESTIMATING CHARTS

These tables are provided for **estimating purposes only**. They should not be used or relied upon for any application without verification of accuracy, suitability, and applicability for the use contemplated, which is the sole responsibility of the user. A final, project-specific design should be prepared by a qualified, licensed, professional Civil Engineer (P.E.) based on actual site conditions. Preparation of these tables did not include consideration or analysis of global slope stability or allowable bearing capacity of foundation soils. These must be reviewed for each project by a qualified Geotechnical Engineer.

There are three tables provided in this guide to help estimate geogrid for different wall loading situations—level backfill, sloping backfill, and surcharges. To estimate geogrid quantities, first look under the **column** appropriate for project soils, determine the height (H) of the proposed wall and read across the **row** (under appropriate soil column) to approximate geogrid type, number of layers, and lengths of each layer.

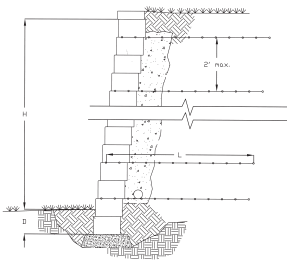
These design charts assume the following conditions:

- Uniform soil conditions
- Stable foundation soils
- Level grade in front of the wall
- No groundwater/water loads
- Slopes and loads behind the wall as shown
- No additional loading behind wall (such as tiered walls, building loads, etc.)

Design standards and properties used to develop these charts were:

- Design methodology - in general accordance with NCMA Design Manual for SRWs
- Unit weight of soil (γ) 120 pcf
- Internal friction angle of soil (ϕ) as shown on charts
- Long term design strength of the geogrid (LTDS)
- **VERSA-Grid VG 3.0 - 1250 lb/ft**
- **VERSA-Grid VG 5.0 - 1875 lb/ft**

LEVEL BACKFILL



Gravel ($\phi = 34^\circ$)

H (feet)	D (feet)	L (feet)	layers	Versa-Grid
4	0.5	3.5	1	VG 3.0
5	0.5	3.5	2	VG 3.0
6	0.5	4.0	2	VG 3.0
7	1.0	5.0	3	VG 3.0
8	1.0	5.5	4	VG 3.0
9	1.0	6.0	4	VG 3.0
10	1.0	6.5	5	VG 3.0
12	1.0	8.0	6	VG 3.0

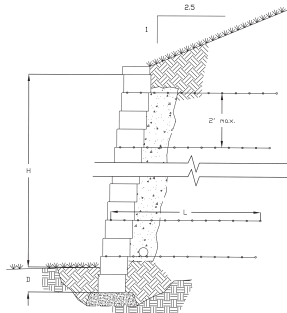
Sand ($\phi = 30^\circ$)

H (feet)	D (feet)	L (feet)	layers	Versa-Grid
4	0.5	4.0	1	VG 3.0
5	0.5	4.0	2	VG 3.0
6	0.5	4.5	2	VG 3.0
7	1.0	5.5	3	VG 3.0
8	1.0	6.0	4	VG 3.0
9	1.0	6.5	5	VG 3.0
10	1.0	7.0	5	VG 3.0
12	1.0	8.5	7	VG 3.0

Clay ($\phi = 28^\circ$)

H (feet)	D (feet)	L (feet)	layers	Versa-Grid
4	0.5	4.0	1	VG 3.0
5	0.5	4.5	2	VG 3.0
6	0.5	5.0	2	VG 3.0
7	1.0	5.5	3	VG 3.0
8	1.0	6.0	4	VG 3.0
9	1.0	6.5	5	VG 3.0
10	1.0	7.0	6	VG 3.0
12	1.0	8.5	7	VG 3.0

SLOPING BACKFILL



Gravel ($\phi = 34^\circ$)

H (feet)	D (feet)	L (feet)	layers	Versa-Grid
4	0.5	4.0	1	VG 3.0
5	0.5	4.0	2	VG 3.0
6	0.5	4.5	3	VG 3.0
7	1.0	5.5	4	VG 3.0
8	1.0	6.0	4	VG 3.0
9	1.0	6.5	5	VG 3.0
10	1.0	7.5	6	VG 3.0
12	1.0	8.5	7	VG 3.0

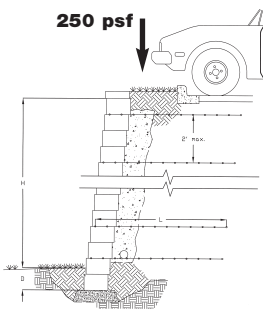
Sand ($\phi = 30^\circ$)

H (feet)	D (feet)	L (feet)	layers	Versa-Grid
4	0.5	4.5	1	VG 3.0
5	0.5	4.5	2	VG 3.0
6	0.5	5.5	3	VG 3.0
7	1.0	6.5	4	VG 3.0
8	1.0	7.0	5	VG 3.0
9	1.0	8.0	6	VG 3.0
10	1.0	8.5	6	VG 3.0
12	1.0	10.0	7	VG 5.0

Clay ($\phi = 28^\circ$)

H (feet)	D (feet)	L (feet)	layers	Versa-Grid
4	0.5	4.5	2	VG 3.0
5	0.5	5.5	2	VG 3.0
6	0.5	6.0	3	VG 3.0
7	1.0	8.0	4	VG 3.0
8	1.0	9.5	5	VG 3.0
9	1.0	11.0	6	VG 3.0
10	1.0	12.0	6	VG 5.0
12	1.0	15.0	7	VG 5.0

SURCHARGE BACKFILL



Gravel ($\phi = 34^\circ$)

H (feet)	D (feet)	L (feet)	layers	Versa-Grid
4	0.5	4.0	2	VG 3.0
5	0.5	4.5	2	VG 3.0
6	0.5	5.0	3	VG 3.0
7	1.0	6.0	4	VG 3.0
8	1.0	6.5	4	VG 3.0
9	1.0	7.0	5	VG 3.0
10	1.0	7.5	5	VG 3.0
12	1.0	9.0	7	VG 3.0

Sand ($\phi = 30^\circ$)

H (feet)	D (feet)	L (feet)	layers	Versa-Grid
4	0.5	4.5	2	VG 3.0
5	0.5	5.5	2	VG 3.0
6	0.5	6.0	3	VG 3.0
7	1.0	7.0	4	VG 3.0
8	1.0	7.5	5	VG 3.0
9	1.0	8.5	5	VG 3.0
10	1.0	9.0	6	VG 3.0
12	1.0	10.0	7	VG 5.0

Clay ($\phi = 28^\circ$)

H (feet)	D (feet)	L (feet)	layers	Versa-Grid
4	0.5	5.5	2	VG 3.0
5	0.5	6.0	2	VG 3.0
6	0.5	6.5	3	VG 3.0
7	1.0	7.5	4	VG 3.0
8	1.0	8.0	5	VG 3.0
9	1.0	9.0	5	VG 3.0
10	1.0	9.5	6	VG 3.0
12	1.0	11.0	7	VG 5.0

*Geogrids with similar LTDS and connection strengths to VERSA-LOK® units can also be estimated using these charts. With some variations, the VERSA-Grid VG 3.0 charts also generally estimate quantities for Miragrid 3XT, Stratagrid 200, and Raugrid 4/2. The charts for VERSA-Grid VG 5.0 generally estimate quantities for Miragrid 5XT, Stratagrid 350, and Raugrid 6/3.

Miragrid is a registered trademark of Nicolon Corporation. • Stratagrid is a registered trademark of Strata Systems, Inc.
Raugrid is a trademark of Lückenhaus Technische Textilien GmbH and Lückenhaus North America, Inc.

SQUARE FOOT™ SPECIFICATIONS

1.01 DESCRIPTION

- A. *Work includes furnishing and installing segmental retaining wall (SRW) units to the lines and grades designated on the project's final construction drawings or as directed by the Architect/Engineer. Also included are furnishing and installing appurtenant materials required for construction of the retaining wall as shown on the construction drawings.*

1.02 REFERENCE STANDARDS

- A. *Segmental Retaining Wall Units*
1. **ASTM C 1372**
 - Standard Specification for Segmental Retaining Wall Units
 2. **ASTM C 140**
 - Standard Test Methods of Sampling and Testing Concrete Masonry Units
- B. *Geosynthetic Reinforcement*
1. **ASTM D 4595**
 - Tensile Properties of Geotextiles by the Wide-Width Strip Method
 2. **ASTM D 5262**
 - Test Method for Evaluating the Unconfined Creep Behavior of Geosynthetics
 3. **GRI:GG1**
 - Single Rib Geogrid Tensile Strength
 4. **GRI:GG5**
 - Geogrid Pullout
- C. *Soils*
1. **ASTM D 698**
 - Moisture Density Relationship for Soils, Standard Method
 2. **ASTM D 422**
 - Gradation of Soils
 3. **ASTM D 424**
 - Atterberg Limits of Soil
- D. *Drainage Pipe*
1. **ASTM D 3034**
 - Specification for Polyvinyl Chloride (PVC) Plastic Pipe
 2. **ASTM D 1248**
 - Specification for Corrugated Plastic Pipe

E. *Engineering Design*

1. "NCMA Design Manual for Segmental Retaining Walls," Second Edition

- F. *Where specifications and reference documents conflict, the Architect/Engineer shall make the final determination of applicable document.*

1.03 SUBMITTALS

- A. **Material Submittals:** *The Contractor shall submit manufacturers' certifications two weeks prior to start of work stating that the SRW units and geosynthetic reinforcement meet the requirements of Section 2 of this specification.*
- B. **Design Submittal:** *The Contractor shall submit two sets of detailed design calculations and final retaining wall plans for approval at least two weeks prior to the beginning of wall construction. All calculations and drawings shall be prepared and sealed by a professional Civil Engineer (P.E.) – (Wall Design Engineer) experienced in SRW design and licensed in the state where the wall is to be built.*

1.04 DELIVERY, STORAGE AND HANDLING

- A. *Contractor shall check materials upon delivery to assure that specified type and grade of materials have been received and proper color and texture of SRW units have been received.*
- B. *Contractor shall prevent excessive mud, wet concrete, epoxies, and like materials that may affix themselves, from coming in contact with materials.*
- C. *Contractor shall store and handle materials in accordance with manufacturer's recommendations.*
- D. *Contractor shall protect materials from damage. Damaged materials shall not be incorporated into the retaining wall.*

SQUARE FOOT™ SPECIFICATIONS

2.01 SEGMENTAL RETAINING WALL UNITS

- A. SRW units shall be machine-formed, Portland Cement concrete blocks specifically designed for retaining wall applications. SRW units currently approved for this project are: Square Foot™ units as manufactured by _____.
- B. Color of SRW units shall be _____.
- C. Finish of SRW units shall be split face.
- D. SRW unit faces shall be of straight geometry.
- E. SRW unit height shall be 8 inches.
- F. SRW units (not including aggregate fill in unit voids) shall provide a minimum weight of 80 psf wall face area.
- G. SRW units shall be interlocked with connection pins, designed with proper setback to provide 8:1 vertical to horizontal batter (a 7 degree cant from vertical) or near vertical (2 degree cant from vertical).
- H. SRW units shall be capable of being erected with the horizontal gap between adjacent units not exceeding 1/8 inch.
- I. SRW units shall be sound and free of cracks or other defects that would interfere with the proper placing of the unit or significantly impair the strength or permanence of the structure. Cracking or excessive chipping may be grounds for rejection. Units showing cracks longer than 1/2" shall not be used within the wall. Units showing chips visible at a distance of 30 feet from the wall shall not be used within the wall.
- J. Concrete used to manufacture SRW units shall have a minimum 28 days compressive strength of 3,000 psi and a maximum moisture absorption rate, by weight, of 8 percent as determined in accordance with ASTM C140. Compressive strength test specimens shall conform to the saw-cut coupon provisions of ASTM C140.
- K. SRW units' molded dimensions shall not differ more than $\pm 1/8$ inch from that specified, in accordance with ASTM C1372.

2.02 SEGMENTAL RETAINING WALL UNIT CONNECTION PINS

- A. SRW units shall be interlocked with VERSA-TUFF® connection pins. The pins shall consist of glass-reinforced nylon made for the expressed use with the SRW units supplied.

2.03 GEOSYNTHETIC REINFORCEMENT

- A. Geosynthetic reinforcement shall consist of geogrids or geotextiles manufactured as a soil reinforcement element. The manufacturers/suppliers of the geosynthetic reinforcement shall have demonstrated construction of similar size and types of segmental retaining walls on previous projects. The geosynthetic type must be approved one week prior to bid opening. Geosynthetic types currently approved for this project are: **VERSA-Grid® Geogrids**.
- B. The type, strength, and placement location of the reinforcing geosynthetic shall be as determined by the Wall Design Engineer, as shown on the final, P.E. sealed retaining wall plans.

2.04 LEVELING PAD

- A. Material for leveling pad shall consist of compacted sand, gravel, or combination thereof (USCS soil types GP, GW, SP, & SW) and shall be a minimum of 6 inches in depth. Lean concrete with a strength of 200 to 300 psi and three inches thick maximum may also be used as a leveling pad material. The leveling pad should extend laterally at least a distance of 6 inches from the toe and heel of the lowermost SRW unit.

2.05 DRAINAGE AGGREGATE

- A. Drainage aggregate shall be angular, clean stone or granular fill meeting the following gradation as determined in accordance with ASTM D422

<u>Sieve Size</u>	<u>Percent Passing</u>
1 inch	100
3/4 inch	75-100
No. 4	0-60
No. 40	0-50
No. 200	0-5

SQUARE FOOT™ SPECIFICATIONS

2.06 DRAINAGE PIPE

- A. The drainage collection pipe shall be a perforated or slotted PVC, or corrugated HDPE pipe. The drainage pipe may be wrapped with a geotextile to function as a filter.
- B. Drainage pipe shall be manufactured in accordance with ASTM D 3034 and/or ASTM D 1248

2.07 REINFORCED (INFILL) SOIL

- A. The reinforced soil material shall be free of debris. Unless otherwise noted on the final, P.E. sealed retaining wall plans prepared by the Wall Design Engineer, the reinforced material shall consist of the inorganic USCS soil types GP, GW, SW, SP, SM meeting the following gradation, as determined in accordance with ASTM D422:

<u>Sieve Size</u>	<u>Percent Passing</u>
4 inches	100
No. 4	20-100
No. 40	0-60
No. 200	0-35

- B. The maximum particle size of poorly-graded gravels (GP) (no fines) should not exceed 3/4 inch unless expressly approved by the Wall Design Engineer and the long-term design strength (LTDS) of the geosynthetic is reduced to account for additional installation damage from particles larger than this maximum.
- C. The plasticity of the fine fraction shall be less than 20.

3.01 SOIL

- A. The following soil parameters, as determined by the Owner's Geotechnical Engineer shall be used for the preparation of the final design:

	Unit Weight (γ) (pcf)	Internal Friction Angle (φ) (degrees)	Cohesion (c)
REINFORCED FILL:			0
RETAINED SOIL:			0
FOUNDATION SOIL:			

(If internal friction angles are not available for the above section, the specifier can provide the USCS soil type classification for the reinforced, retained, and foundation soils and/or attach the geotechnical investigation report for this project.)

- B. Should the actual soil conditions observed during construction differ from those assumed for the design, design shall be reviewed by the Wall Design Engineer at the Owner's Geotechnical Engineer's direction.

3.02 DESIGN

- A. The design analysis for the final, P.E.-sealed retaining wall plans prepared by the Wall Design Engineer shall consider the external stability against sliding and overturning, internal stability, and facial stability of the reinforced soil mass and shall be in accordance with acceptable engineering practice and these specifications. The internal and external stability analysis shall be performed in accordance with the "NCMA Design Manual for Segmental Retaining Walls", using the recommended minimum factors of safety in this manual.
- B. External stability analysis for bearing capacity, global stability, and total and differential settlement shall be the responsibility of the Owner and the Owner's Geotechnical Engineer. The Geotechnical Engineer shall perform bearing capacity, settlement estimates, and global stability analysis based on the final wall design provided by the Wall Design Engineer and coordinate any required changes with Wall Design Engineer.
- C. While vertical spacing between geogrid layers may vary, it shall not exceed 2.0 feet maximum in the wall design.

SQUARE FOOT™ SPECIFICATIONS

- D. *The geosynthetic placement in the wall design shall have 100-percent continuous coverage parallel to the wall face. Gapping between horizontally adjacent layers of geosynthetic (partial coverage) will not be allowed.*

4.01 INSPECTION

- A. *The Owner or Owner's Representative is responsible for verifying that the Contractor meets all the requirements of the specification. This includes all submittals for materials and design, qualifications, and proper installation of wall system.*
- B. *Contractor's field construction supervisor shall have demonstrated experience and be qualified to direct all work at the site.*

4.02 EXCAVATION

- A. *Contractor shall excavate to the lines and grades shown on the project grading plans. Contractor shall take precautions to minimize over-excavation. Over-excavation shall be filled with compacted infill material, or as directed by the Engineer/Architect, at the Contractor's expense.*
- B. *Contractor shall verify location of existing structures and utilities prior to excavation. Contractor shall ensure all surrounding structures are protected from the effects of wall excavation. Excavation support, if required, is the responsibility of the Contractor.*

4.03 FOUNDATION PREPARATION

- A. *Following the excavation, the foundation soil shall be examined by the Owner's Engineer to assure actual foundation soil strength meets or exceeds the assumed design bearing strength. Soils not meeting the required strength shall be removed and replaced with infill soils, as directed by the Owner's Engineer.*
- B. *Foundation soil shall be proofrolled and compacted to 95 percent standard Proctor density and inspected by the Owner's Engineer prior to placement of leveling pad materials.*

4.04 LEVELING PAD CONSTRUCTION

- A. *Leveling pad shall be placed as shown on the final, P.E.-sealed retaining wall plans with a minimum thickness of 6 inches. The leveling pad should extend laterally at least a distance of 6 inches from the toe and heel of the lower most SRW unit.*
- B. *Granular leveling pad material shall be compacted to provide a firm, level bearing surface on which to place the first course of units. Well-graded sand can be used to smooth the top 1/4- to 1/2-inch of the leveling pad. Compaction will be with mechanical plate compactors to achieve 95-percent of maximum standard Proctor density (ASTM D 698).*

4.05 SRW UNIT INSTALLATION

- A. *All SRW units shall be installed at the proper elevation and orientation as shown on the final, P.E.-sealed retaining wall plans and details as directed by the Wall Design Engineer. The SRW units shall be installed in general accordance with the manufacturer's recommendations. The specifications and drawings shall govern in any conflict between the two requirements.*
- B. *First course of SRW units shall be placed on the leveling pad. The units shall be leveled side-to-side, front-to-rear and with adjacent units, and aligned to ensure intimate contact with the leveling pad. The first course is the most important to ensure accurate and acceptable results. No gaps shall be left between the front of adjacent units. Alignment may be done by means of a string line or offset from base line to the back of the units.*
- C. *The voids (cores) of the base course units shall be filled with crushed gravel. The cores of additional courses shall be filled with free-draining aggregate.*
- D. *All excess debris shall be cleaned from top of units and the next course of units installed on top of the units below.*
- E. *Two VERSA-TUFF® connection pins shall be inserted through the pin holes of each upper course unit into receiving slots in lower course units. Pins shall be fully seated in the pin slot below. Units shall be pushed forward to remove any looseness in the unit-to-unit connection.*
- F. *Prior to placement of next course, the level and alignment of the units shall be checked and corrected, where needed.*

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- G. *Layout of curves and corners shall be installed in accordance with the wall plan details or in general accordance with SRW manufacturer's installation guidelines. Walls meeting at corners shall be interlocked by overlapping successive courses.*
- H. *Procedures "C" through "F" shall be repeated until reaching top of wall, just below the height of the cap units. Geosynthetic reinforcement, drainage materials, and reinforced backfill shall be placed in sequence with unit installation as described in Section 4.06, 4.07, and 4.08.*

4.06 GEOSYNTHETIC REINFORCEMENT PLACEMENT

- A. *All geosynthetic reinforcement shall be installed at the proper elevation and orientation as shown on the final, P.E.-sealed retaining wall plan profiles and details, or as directed by the Wall Design Engineer.*
- B. *At the elevations shown on the final plans, (after the units, drainage material, and backfill have been placed to this elevation) the geosynthetic reinforcement shall be laid horizontally on compacted infill and on top of the concrete SRW units. It shall be placed to within one inch of the front face of the unit below. Embedment of the geosynthetic in the SRW units shall be consistent with SRW manufacturer's recommendations. Correct orientation of the geosynthetic reinforcement shall be verified by the Contractor to be in accordance with the geosynthetic manufacturer's recommendations. The highest strength direction of the geosynthetic must be perpendicular to the wall face.*
- C. *Geosynthetic reinforcement layers shall be one continuous piece for their entire embedment length. Splicing of the geosynthetic in the design strength direction (perpendicular to the wall face) shall not be permitted. Along the length of the wall (parallel to the face), horizontally adjacent sections of geosynthetic reinforcement shall be butted in a manner to assure 100-percent coverage parallel to the wall face.*
- D. *Tracked construction equipment shall not be operated directly on the geosynthetic reinforcement. A minimum of 6 inches of backfill is required prior to operation of tracked vehicles over the geosynthetic. Turning should be kept to a minimum. Rubber-tired equipment may pass over the geosynthetic reinforcement at slow speeds (less than 5 mph).*

- E. *The geosynthetic reinforcement shall be free of wrinkles prior to placement of soil fill. The nominal tension shall be applied to the reinforcement and secured in place with staples, stakes, or by hand tensioning until reinforcement is covered by 6 inches of fill.*

4.07 DRAINAGE MATERIALS

- A. *Drainage aggregate shall be installed to the line, grades, and sections shown on the final P.E. sealed retaining wall plans. Drainage aggregate shall be placed to the minimum thickness shown on the construction plans between and behind units (a minimum of 1 cubic foot for each exposed square foot of wall face unless otherwise noted on the final wall plans).*
- B. *Drainage collection pipes shall be installed to maintain gravity flow of water to outside the reinforced soil zone. The drainage collection pipe shall daylight into a storm sewer manhole or along a slope at an elevation lower than the lowest point of the pipe within the aggregate drain.*

4.08 BACKFILL PLACEMENT

- A. *The reinforced backfill shall be placed as shown in the final wall plans in the maximum compacted lift thickness of 10 inches and shall be compacted to a minimum of 95-percent of standard Proctor density (ASTM D 698) at a moisture content within 2-percent of optimum. The backfill shall be placed and spread in such a manner as to eliminate wrinkles or movement of the geosynthetic reinforcement and the SRW units.*
- B. *Only hand-operated compaction equipment shall be allowed within 3 feet of the back of the wall unit. Compaction within the 3 feet behind the wall unit shall be achieved by at least three (3) passes of a lightweight mechanical tamper, plate, or roller.*
- C. *At the end of each day's operation, the Contractor shall slope the last level of backfill away from the wall facing and reinforced backfill to direct water runoff away from the wall face.*
- D. *At completion of wall construction, backfill shall be placed level with final top-of-wall elevation. If final grading, paving, landscaping, and/or storm drainage installation adjacent to the wall is not placed*

SQUARE FOOT™ SPECIFICATIONS

immediately after wall completion, temporary grading and drainage shall be provided to ensure water runoff is not directed at the wall nor allowed to collect or pond behind the wall until final construction adjacent to the wall is completed.

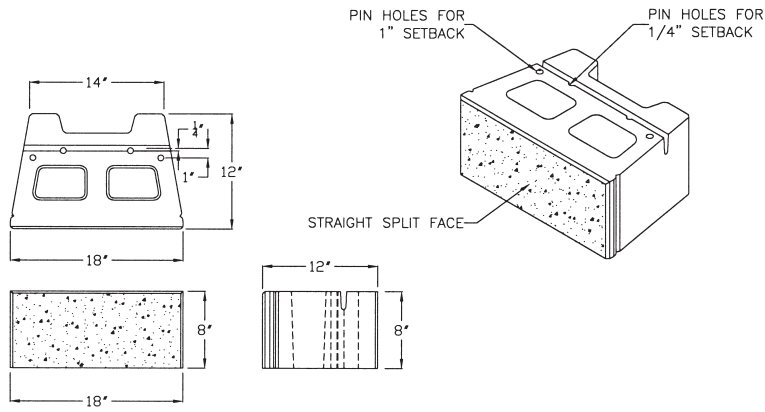
4.09 SRW CAPS

- A. *SRW caps shall be properly aligned and glued to underlying units with VERSA-LOK® Concrete Adhesive, a flexible, high-strength concrete adhesive. Rigid adhesive or mortar are not acceptable.*
- B. *Caps shall overhang the top course of units by 3/4- to 1-inch. Slight variation in overhang is allowed to correct alignment at the top of the wall.*

4.11 CONSTRUCTION ADJACENT TO COMPLETED WALL

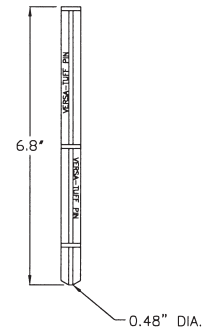
- A. *The Owner or Owner's Representative is responsible for ensuring that construction by others adjacent to the wall does not disturb the wall or place temporary construction loads on the wall that exceed design loads, including loads such as water pressure, temporary grades, or equipment loading. Heavy paving or grading equipment shall be kept a minimum of 3 feet behind the back of the wall face. Equipment with wheel loads in excess of 150 psf live load shall not be operated within 10 feet of the face of the retaining wall during construction adjacent to the wall. Care should be taken by the General Contractor to ensure water runoff is directed away from the wall structure until final grading and surface drainage collection systems are completed.*

CONSTRUCTION DETAILS



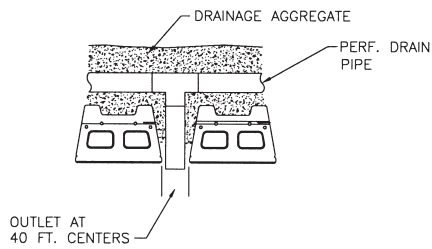
VERSA-LOK SQUARE FOOT UNIT

UNIT DIMENSIONS
SCALE: NONE



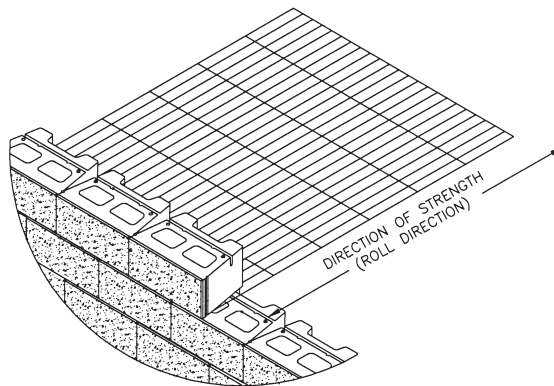
VERSA-TUFF PIN

PIN DIMENSIONS
SCALE: NONE



DRAIN DETAIL

WALLS OVER 3'
SCALE: NONE

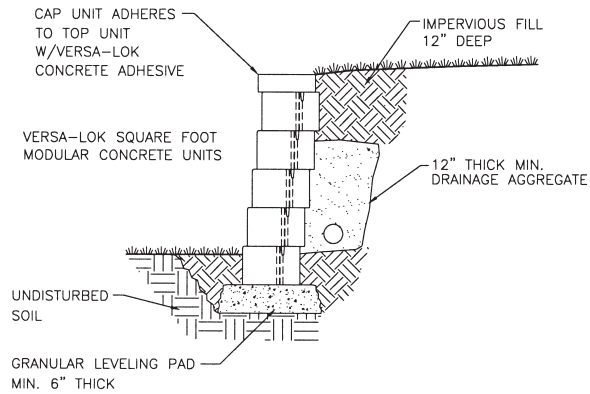


- NOTE:
1. FOLLOW GEOSYNTHETIC GRID MANUFACTURER'S INSTALLATION INSTRUCTIONS AND SPECIFICATIONS
 2. GEOGRID LENGTH AND ELEVATION PLACEMENT SHALL BE DETERMINED BY PROJECT ENGINEER

GEOSYNTHETIC INSTALLATION DETAIL

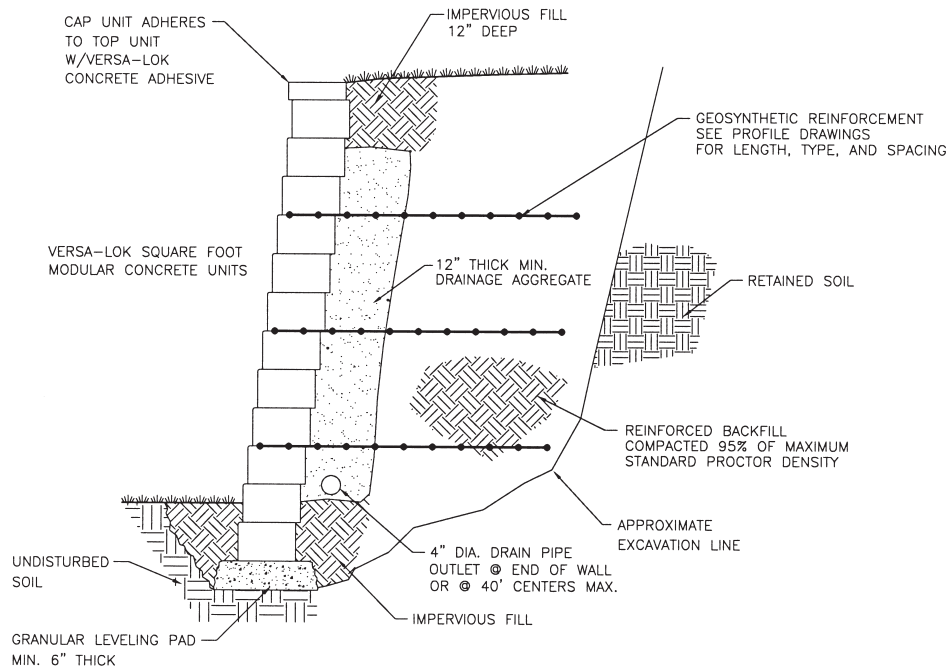
SCALE: NONE

CONSTRUCTION DETAILS



TYPICAL SECTION – UNREINFORCED RETAINING WALL

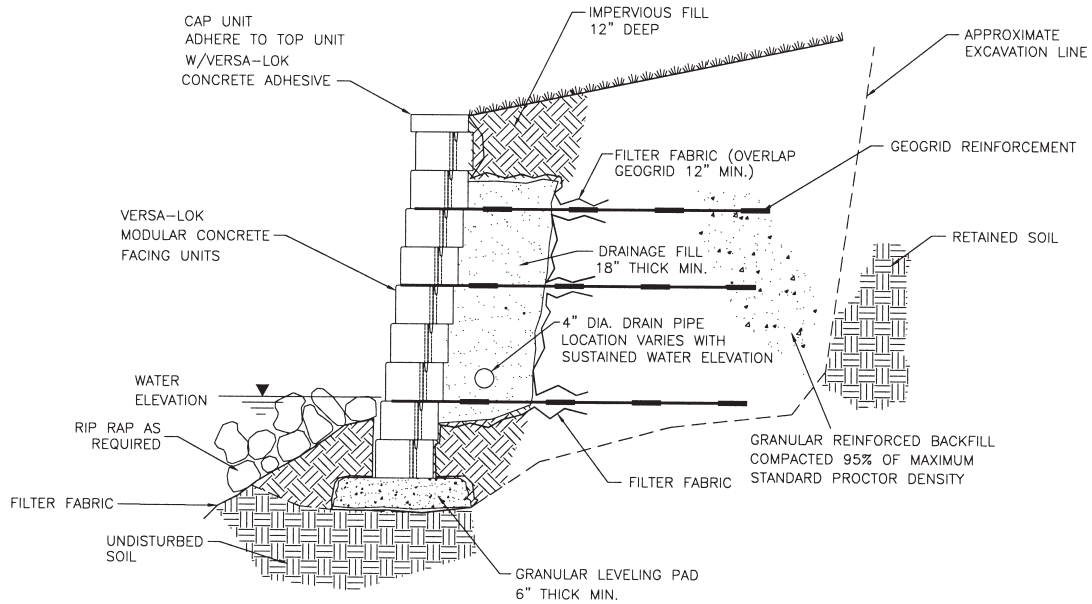
SCALE: NONE



TYPICAL SECTION – REINFORCED RETAINING WALL

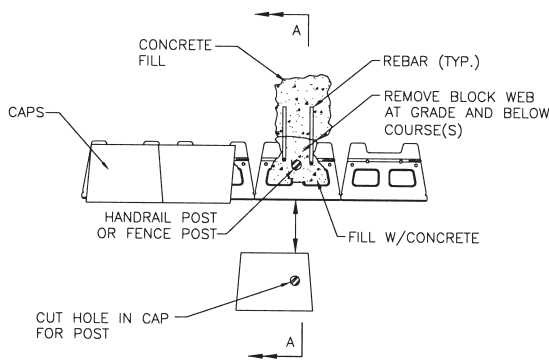
SCALE: NONE

CONSTRUCTION DETAILS



TYPICAL SECTION-REINFORCED SHORELINE WALL

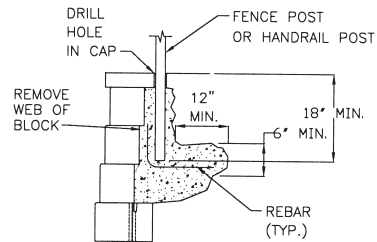
SCALE: NONE



POST DETAIL- PLAN VIEW

TYPICAL HANDRAIL AND/OR FENCE POST

SCALE: NONE

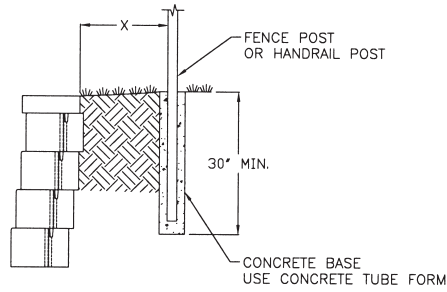


POST DETAIL- SECTION A-A

TYPICAL HANDRAIL AND/OR FENCE POST

SCALE: NONE

CONSTRUCTION DETAILS

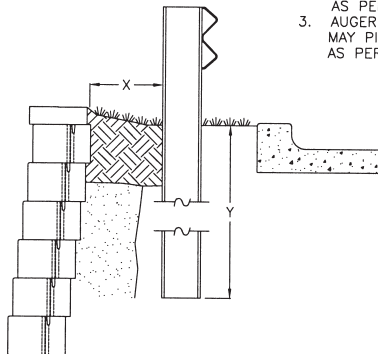


POST DETAIL

TYPICAL HANDRAIL AND/OR FENCE POST
SCALE: NONE

GUARD RAIL NOTES:

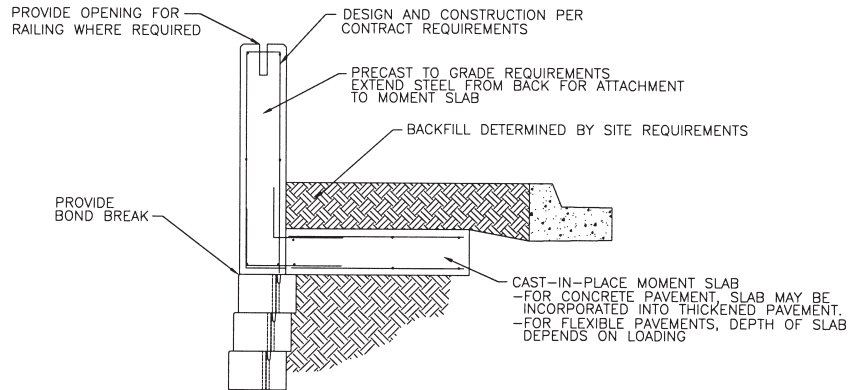
1. DISTANCE X AND Y TO BE DETERMINED BY ENGINEER BASED ON SOIL AND LOADING CONDITIONS.
2. INSTALL H-PILE OR WOOD POST AS PER MANUFACTURER'S RECOMMENDATIONS.
3. AUGERING OR DRIVING OF POST MAY PIERCE UPPER LAYER OF GEOSYNTHETIC AS PER ENGINEER'S DESIGN.



GUARD RAIL DETAIL

TYPICAL GUARD RAIL
SCALE: NONE

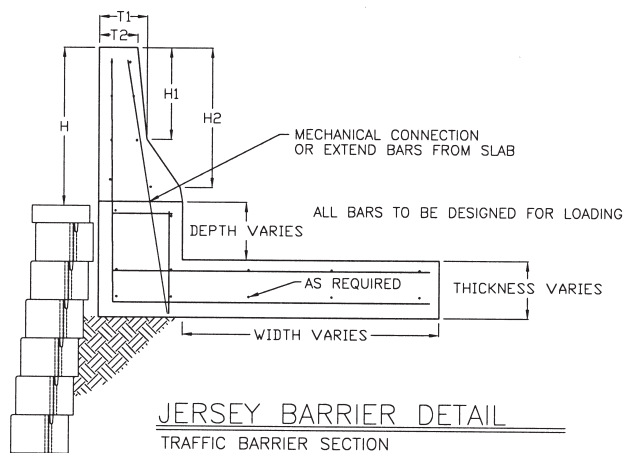
CONSTRUCTION DETAILS



COPING DETAIL
TRAFFIC BARRIER SECTION
SCALE: NONE

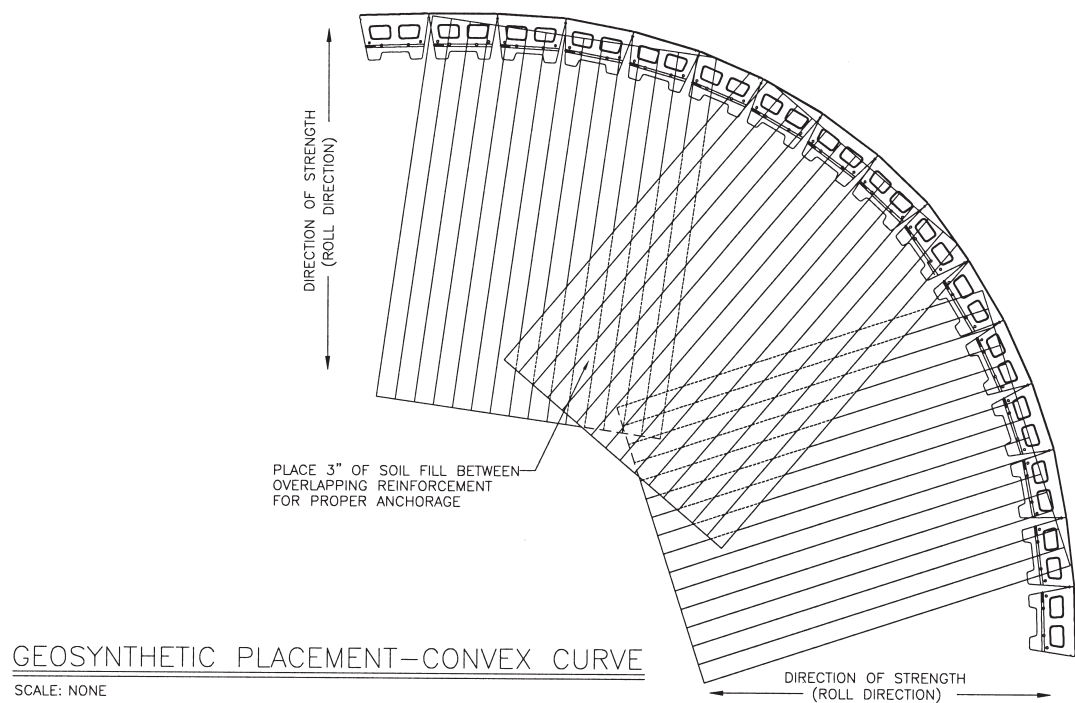
BARRIER NOTES:

1. BARRIERS MAY BE SET ON TOP OF OR BEHIND WALL.
2. BARRIERS ON TOP OF WALL SHALL BE DESIGNED TO ACCOMMODATE DIFFERENTIAL MOVEMENT OF RETAINING WALL.
3. STEEL DESIGN AND COPING GEOMETRY WILL VARY WITH SITE LOADINGS. DESIGN MUST BE PROJECT SPECIFIC.
4. DURING PLACEMENT OF CONCRETE, PRECAUTIONS SHOULD BE TAKEN TO REDUCE LATERAL PRESSURES ON THE VERSA-LOK WALL. FORMING AND/OR TEMPORARY BRACING MAY BE REQUIRED.



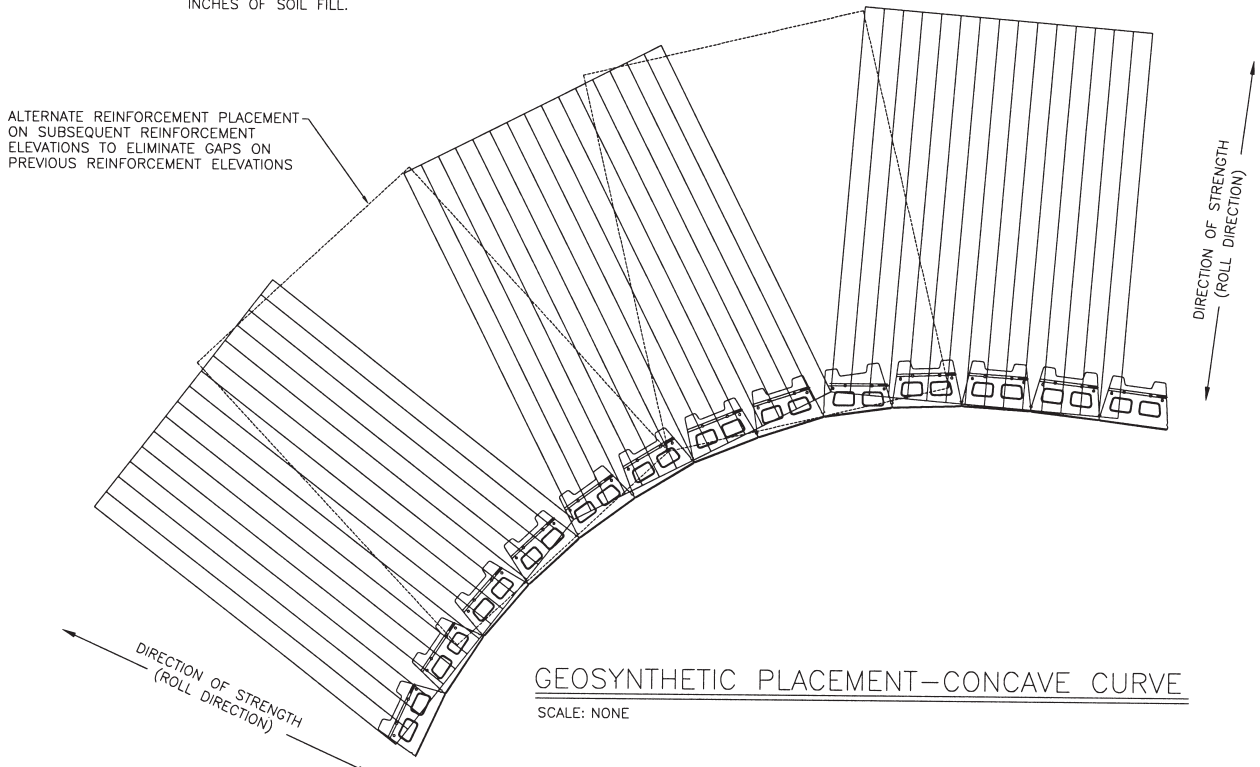
JERSEY BARRIER DETAIL
TRAFFIC BARRIER SECTION
SCALE: NONE

CONSTRUCTION DETAILS



GENERAL NOTES FOR GEOSYNTHETIC REINFORCEMENT

1. AT THE ELEVATIONS SHOWN ON THE CONSTRUCTION PLANS, THE GEOSYNTHETIC SHALL BE LAID HORIZONTALLY ON COMPACTED SOIL FILL AND PLACED ON TO THE VERSA-LOK UNITS.
2. CORRECT ORIENTATION (ROLL DIRECTION) OF THE GEOSYNTHETIC SHALL BE VERIFIED BY THE CONTRACTOR TO BE IN ACCORDANCE WITH THE GEOSYNTHETIC MANUFACTURER'S RECOMMENDATIONS.
3. THE GEOSYNTHETIC SHALL BE IN TENSION AND FREE OF WRINKLES PRIOR TO PLACEMENT OF SOIL FILL.
4. NOMINAL TENSION SHALL BE APPLIED TO THE GEOSYNTHETIC AND SECURED IN PLACE WITH STAPLES, STAKES OR BY HAND TENSIONING UNTIL THE GEOSYNTHETIC IS COVERED BY SIX INCHES OF SOIL FILL.





Square Foot & Installation Guidelines

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Products shown may be covered by one or more of the following:

U.S. Patent D319,885, U.S. Patent D321,060, U.S. Patent D341,215, U.S. Patent D346,667, U.S. Patent D378,702, U.S. Patent D391,376, U.S. Patent D430,680, U.S. Patent D435,302, U.S. Patent D439,678, U.S. Patent D452,332, U.S. Patent D458,387, U.S. Patent 6,488,448, U.S. Patent 6,960,048, U.S. Patent 7,229,235, U.S. Patent 7,244,079, U.S. Patent D552,258, U.S. Patent D555,810 and other U.S. patents pending; Canadian Industrial Design Registration No. 63929, No. 71472, No. 73910, No. 73911, No. 73912, No. 77816, No. 79058, No. 82288, and No. 89084. I.C.B.O. No. 4625, Canadian Patent 2,313,061

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